

Esfuerzos internos en viga Gerber

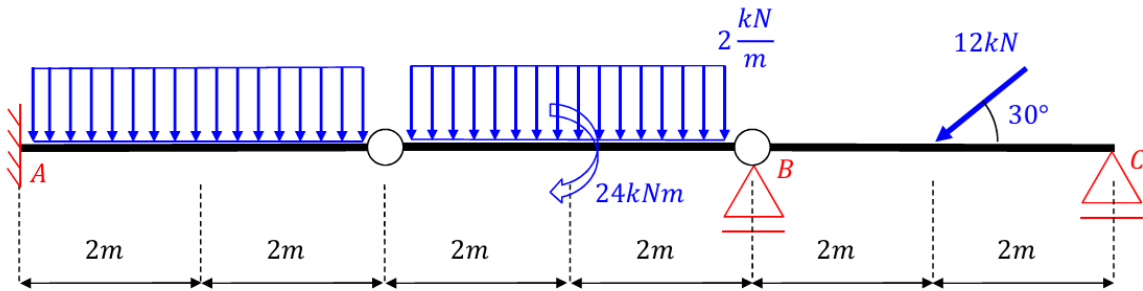
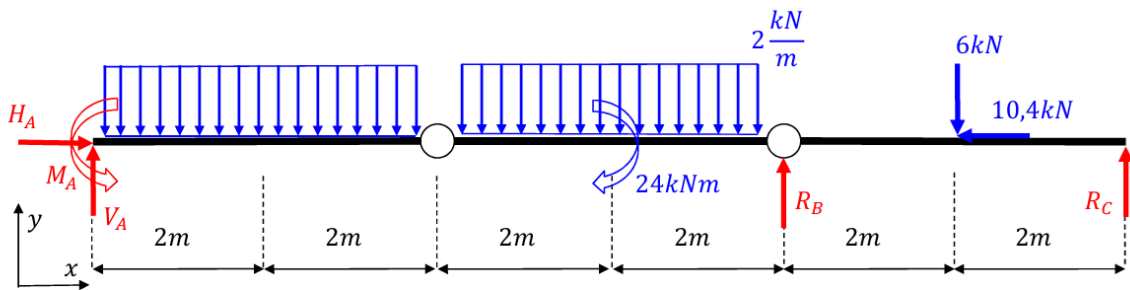


Diagrama de Cuerpo libre



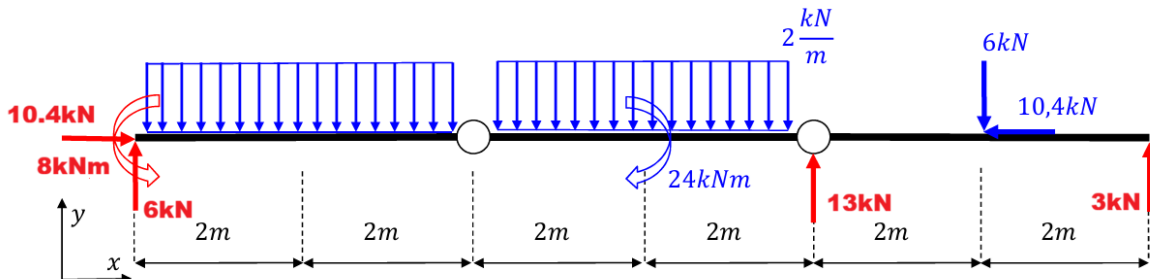
Reacciones de vínculos

$$\sum F_x = H_A - 10,4kN = 0 \rightarrow H_A = 10,4kN \qquad \sum M_{S_3}^{A_{23}} = R_C \cdot 4m - 6kN \cdot 2m = 0 \rightarrow R_C = 3kN$$

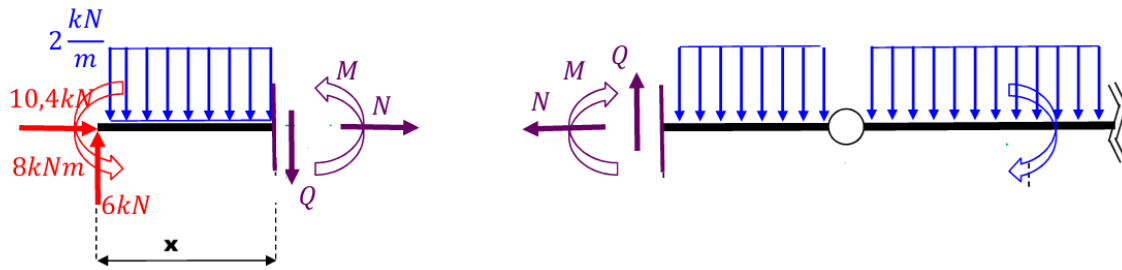
$$\sum M_{S_2+S_3}^{A_{12}} = R_C \cdot 8m - 6kN \cdot 6m - 24kNm - 2 \frac{kN}{m} \cdot 4m \cdot 2m + R_B \cdot 4m = 0 \rightarrow R_B = 13kN$$

$$\sum F_y = V_A - 2 \frac{kN}{m} \cdot 8m + R_B + R_C - 6kN = 0 \rightarrow V_A = 6kN$$

$$\sum M_{S_1}^{A_{12}} = -V_A \cdot 4m + 2 \frac{kN}{m} \cdot 4m \cdot 2m + M_A = 0 \rightarrow M_A = 8kNm$$



Ley de variación de esfuerzos



$$N(x) = -10.4 kN$$

$$Q(x) = 6 kN - \frac{2 kN}{m} \cdot x$$

$$M(x) = -8 kNm + 6 kN \cdot x - \frac{2 kN}{m} \cdot x \cdot \frac{x}{2}$$

Planteo de equilibrio para $x=2m$

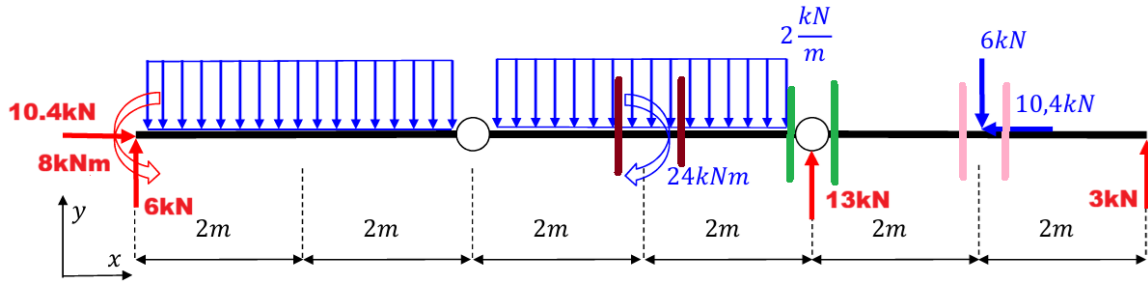
$$\sum F_x = 0 \quad 10.4 kN + N = 0 \quad N = -10.4 kN$$

$$\sum F_y = 0 \quad 6 kN - \frac{2 kN}{m} \cdot 2m - Q = 0 \quad Q = 2 kN$$

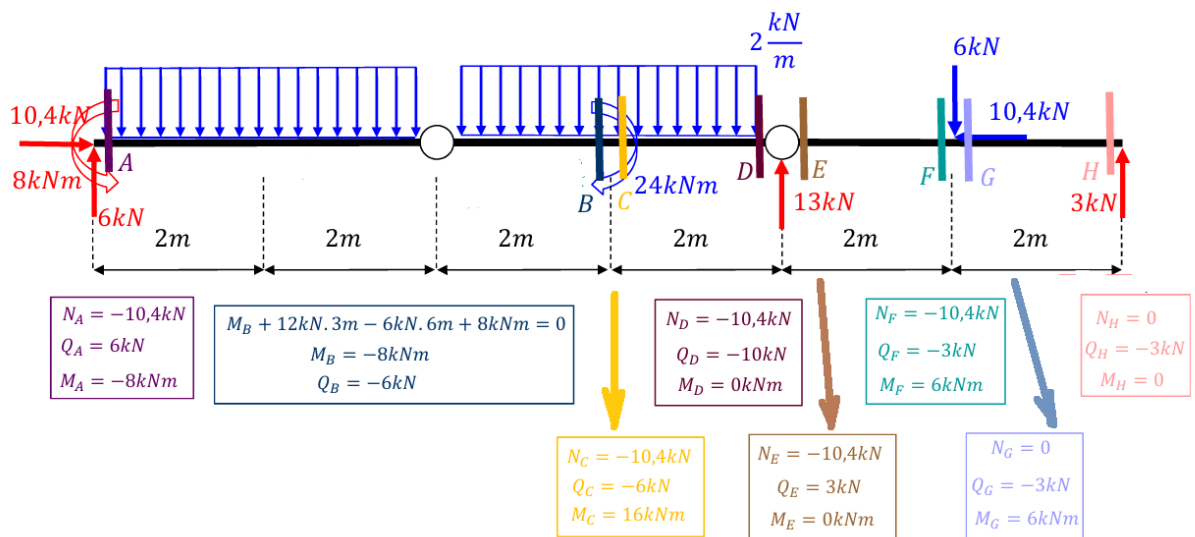
$$\sum M(2m) = 0 \quad -8 kNm + 6 kN \cdot 2m - \frac{2 kN}{m} \cdot 2m \cdot 1m + M = 0$$

$$M = 0 kNm$$

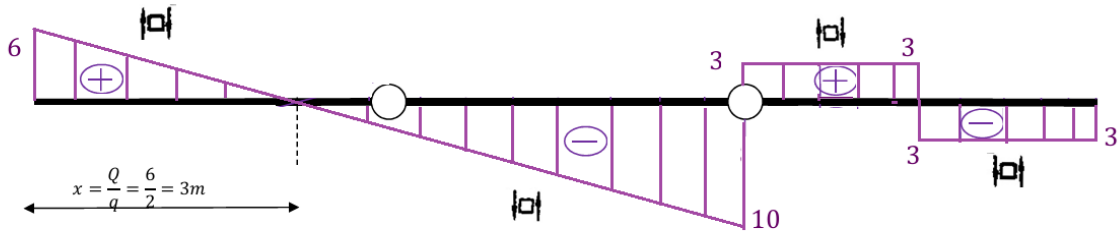
¿Qué produce una discontinuidad en los diagramas de esfuerzos?



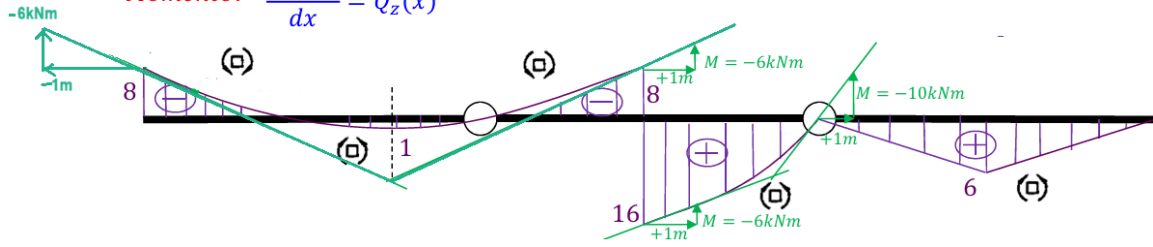
- Cargas puntuales (Fuerzas y Momentos)
- Cambio en la dirección del eje de la estructura



Corte: $\frac{dQ_z(x)}{dx} = -q_z(x)$



Momento: $\frac{dM(x)}{dx} = Q_z(x)$



Normal: $\frac{dN(x)}{dx} = -q_x(x)$

